

Given Properties

Area of Reinforcing Steel	$A_{sprime} := 0.4 \text{in}^2$
Area of Strand	$A_p := 3 \cdot 0.153 \text{in}^2 = 0.459 \cdot \text{in}^2$
Compressive Strength of Concrete at 3 days	$f_{c_3} := 5 \cdot \text{ksi}$
Compressive Strength of Concrete at 28 days	$f_{c_{28}} := 8 \text{ksi}$
Modulus of Elasticity at 3 days	$E_{c_3} := 57 \text{ksi} \sqrt{\frac{f_{c_3}}{\text{psi}}} = 4030.509 \cdot \text{ksi}$
Modulus of Elasticity at 28 days	$E_{c_{28}} := 57 \text{ksi} \sqrt{\frac{f_{c_{28}}}{\text{psi}}} = 5098.235 \cdot \text{ksi}$
Modulus of Elasticity of Steel	$E_s := 29000 \text{ksi}$
Unit Weight of Concrete	$\gamma_c := 0.07 \frac{\text{lb}}{\text{in}^3}$
Unit Weight of Steel	$\gamma_s := 490 \frac{\text{lb}}{\text{ft}^3}$

Section Properties

i := 1..5

Width:

Height:

Area:

Moment of Inertia:

$$b_1 := 8\text{in}$$

$$h_1 := 3\text{in}$$

$$A_1 := b_1 \cdot h_1 = 24 \cdot \text{in}^2$$

$$I_1 := b_1 \cdot \frac{(h_1)^3}{12} = 18 \cdot \text{in}^4$$

$$b_2 := 3\text{in}$$

$$h_2 := 10.5\text{in}$$

$$A_2 := b_2 h_2 = 31.5 \cdot \text{in}^2$$

$$I_2 := b_2 \cdot \frac{(h_2)^3}{12} = 289.406 \cdot \text{in}^4$$

$$b_3 := 8\text{in}$$

$$h_3 := 2.5\text{in}$$

$$A_3 := b_3 h_3 = 20 \cdot \text{in}^2$$

$$I_3 := b_3 \cdot \frac{(h_3)^3}{12} = 10.417 \cdot \text{in}^4$$

$$H := h_1 + h_2 + h_3 = 16 \cdot \text{in}$$

$$A_{\text{concrete}} := (A_1 - A_{\text{prime}}) + A_2 + (A_3 - A_{\text{p}}) = 74.641 \cdot \text{in}^2$$

Centroid:

$$y_1 := h_3 + h_2 + \left(\frac{h_1}{2}\right) = 14.5 \cdot \text{in}$$

$$y_2 := h_3 + \frac{h_2}{2} = 7.75 \cdot \text{in}$$

$$y_3 := \frac{h_3}{2} = 1.25 \cdot \text{in}$$

$$y_4 := \frac{h_3}{2} = 1.25 \cdot \text{in}$$

$$y_5 := h_3 + h_2 + \frac{h_1}{2} = 14.5 \cdot \text{in}$$

Transformed section at 3 days

$$n_3 := \frac{E_s}{E_{c_3}} = 7.195$$

$$A_4 := (n_3 - 1) \cdot A_{s\text{prime}} = 2.478 \cdot \text{in}^2$$

$$A_5 := (n_3 - 1) \cdot A_p = 2.844 \cdot \text{in}^2$$

$$A_{tr_3} := \sum_{i=1}^5 A_i = 80.822 \cdot \text{in}^2$$

$$y_{bar3} := \frac{\left[\sum_{i=1}^5 (A_i \cdot y_i) \right]}{\left(\sum_{i=1}^5 A_i \right)} = 8.184 \cdot \text{in}$$

$$d_1 := y_{bar3} - y_1 = -6.316 \cdot \text{in}$$

$$I_4 := 0$$

$$I_5 := 0$$

$$d_2 := y_{bar3} - y_2 = 0.434 \cdot \text{in}$$

$$d_3 := y_{bar3} - y_3 = 6.934 \cdot \text{in}$$

$$d_4 := y_{bar3} - y_4 = 6.934 \cdot \text{in}$$

$$d_5 := y_{bar3} - y_5 = -6.316 \cdot \text{in}$$

$$I_{tr_3} := \sum_{i=1}^5 \left[I_i + A_i \cdot (d_i)^2 \right] = 2475 \cdot \text{in}^4$$

Transformed Section at 28 days

$$n_{28} := \frac{E_s}{E_{c_{28}}} = 5.688$$

$$A_4 := (n_{28} - 1) \cdot A_{sprime} = 1.875 \cdot \text{in}^2$$

$$A_5 := (n_{28}) \cdot A_p = 2.611 \cdot \text{in}^2$$

$$A_{tr_{28}} := \sum_{i=1}^5 A_i = 79.986 \cdot \text{in}^2$$

$$ybar_{28} := \frac{\left[\sum_{i=1}^5 (A_i \cdot y_i) \right]}{\left(\sum_{i=1}^5 A_i \right)} = 8.218 \cdot \text{in}$$

$$d_1 := ybar_{28} - y_1 = -6.282 \cdot \text{in}$$

$$d_2 := ybar_{28} - y_2 = 0.468 \cdot \text{in}$$

$$d_3 := ybar_{28} - y_3 = 6.968 \cdot \text{in}$$

$$d_4 := ybar_{28} - y_4 = 6.968 \cdot \text{in}$$

$$d_5 := ybar_{28} - y_5 = -6.282 \cdot \text{in}$$

$$I_{tr_{28}} := \sum_{i=1}^5 \left[I_i + A_i \cdot (d_i)^2 \right] = 2437 \cdot \text{in}^4$$

Stresses at Release

Release: $f_{pi} := 174 \text{ksi}$ $F_{pi} := f_{pi} \cdot A_p = 79.866 \cdot \text{kip}$

Cracking: $f_{cr} := 180 \text{ksi}$ $F_{cr} := f_{cr} \cdot A_p = 82.62 \cdot \text{kip}$

Ultimate: $f_u := 265 \text{ksi}$ $F_u := f_u \cdot A_p = 121.635 \cdot \text{kip}$

$$H_{ww} := \sum_{i=1}^3 h_i = 16 \cdot \text{in}$$

$$e_{ww} := y_{bar3} - y_3 = 6.934 \cdot \text{in}$$

axial stress $\sigma_a := \frac{F_{pi}}{A_{tr3}} = 988.176 \cdot \text{psi}$

flexural stress $\sigma_f := \frac{(F_{pi} \cdot e) \cdot y_{bar3}}{I_{tr3}} = 1.831 \times 10^3 \cdot \text{psi}$

$\sigma_t := \sigma_a - \sigma_f = -842.829 \cdot \text{psi}$ stress at top

$\sigma_b := \sigma_a + \sigma_f = 2.819 \times 10^3 \cdot \text{psi}$ stress at bottom

Cracking Capacity

$$\omega_{sw} := (A_{gconcrete} \cdot \gamma_c) + (\gamma_s \cdot A_p) + (\gamma_s \cdot A_{sprime}) = 65.621 \cdot \frac{\text{lb}}{\text{ft}}$$

$$L := 20 \text{ft}$$

$$M_{sw} := \frac{\omega_{sw} \cdot L^2}{8} = 3.281 \cdot \text{ft} \cdot \text{kip}$$

$$\sigma_{sw} := M_{sw} \cdot \frac{y_{bar28}}{I_{tr28}} = 132.773 \cdot \text{psi}$$

$$M_{LL} := 1 \text{kip} \cdot \text{in}$$

Given

$$f_{cr} := 7.5 \text{psi} \sqrt{\frac{f_{c28}}{\text{psi}}} = 670.82 \cdot \text{psi}$$

$$-\sigma_a + \sigma_{sw} - \sigma_f + \frac{M_{LL} \cdot y_{bar28}}{I_{tr28}} = f_{cr}$$

$$M_{LL} := \text{Minerr}(M_{LL})$$

$$P_{cr} := \frac{2 \cdot (M_{LL})}{8 \text{ft}} = 20.741 \cdot \text{kip}$$

$$M_{LL} = 82.964 \cdot \text{kip} \cdot \text{ft}$$

Ultimate Capacity

$$d := y_1 = 14.5 \cdot \text{in}$$

$$d_{\text{prime}} := y_4 = 1.25 \cdot \text{in}$$

$$\varepsilon := 0.003$$

$$f_y := 60 \text{ksi}$$

$$f_p := 265 \text{ksi}$$

$$\beta := \begin{cases} 0.85 & \text{if } f_{c_{28}} \leq 4000 \text{psi} \\ \left[0.85 - \left[0.05 \cdot \frac{(f_{c_{28}} - 4000 \text{psi})}{1000 \text{psi}} \right] \right] & \text{if } 4000 \text{psi} < f_{c_{28}} < 8000 \text{psi} \\ 0.65 & \text{if } f_{c_{28}} \geq 8000 \text{psi} \end{cases}$$

$$c := 1 \text{in}$$

Given

$$(0.85 \cdot f_{c_{28}} \cdot \beta \cdot c \cdot b_1) + \min \left[A_{\text{prime}} \cdot \varepsilon \cdot \left(\frac{c - d_{\text{prime}}}{c} \right) \cdot E_s, f_y \cdot A_{\text{prime}} \right] - A_p \cdot f_p = 0$$

$$c := \text{Minerr}(c)$$

$$c = 2.883 \cdot \text{in}$$

$$C_c := 0.85 \cdot f_{c_{28}} \cdot \beta \cdot c \cdot b_1 = 101.926 \cdot \text{kip}$$

$$C_s := A_{\text{prime}} \cdot E_s \cdot \varepsilon \cdot \left[\frac{(c - y_4)}{c} \right] = 19.709 \cdot \text{kip}$$

$$T := A_p \cdot f_p = 121.635 \cdot \text{kip}$$

$$M_n := f_p \cdot A_p \cdot [d - (\beta \cdot c \cdot 0.5)] + C_s \cdot (\beta \cdot c \cdot 0.5 - d_{\text{prime}}) = 136.965 \cdot \text{ft} \cdot \text{kip}$$

$$P_n := \frac{(M_n - M_{\text{sw}}) \cdot 2}{8 \text{ft}} = 33.421 \cdot \text{kip}$$

Shear Capacity

$$x := 0.1\text{in}, 0.5\text{in}.. 90\text{in}$$

Shear Properties

$$A_v := 0.04\text{in}^2 \quad b_w := 3\text{in} \quad \lambda := 0.75 \quad f_{ym} := 65\text{ksi}$$

$$H = 16\text{-in} \quad l_d := 4\text{in} \quad S_x := 4\text{in}$$

$$d_c := \max(y_s, 0.8H) = 14.5\text{-in}$$

$$V_u(x) := \frac{P_n}{2} + \frac{\omega_{sw} \cdot L}{2} - \omega_{sw} \cdot (x) \quad V_u(0) = 17.367\text{-kip}$$

Concrete Shear Capacity

$$f_{pc}(x) := \begin{cases} \frac{f_p}{l_d} \cdot x & \text{if } 0 < x < l_d \\ f_p & \text{otherwise} \end{cases}$$

$$M_{sw}(x) := \frac{\omega_{sw} \cdot L \cdot (x)}{2} - \frac{\omega_{sw} \cdot (x^2)}{2}$$

$$f_d(x) := \frac{(M_{sw}(x) \cdot y_{bar28})}{I_{tr28}}$$

$$M_{max}(x) := \frac{P_n}{2} \cdot (x)$$

$$f_{pe} := \frac{F_{pi}}{A_{tr28}} + \frac{(F_{pi} \cdot e \cdot y_{bar28})}{I_{tr28}}$$

$$M_{cre}(x) := \left(\frac{I_{tr28}}{y_{bar28}} \right) \cdot \left(6\text{psi} \cdot \lambda \cdot \sqrt{\frac{f_{c28}}{\text{psi}}} + f_{pe} - f_d(x) \right)$$

$$V_d(x) := \frac{\omega_{sw} \cdot L}{2} - \omega_{sw} \cdot (x)$$

$$V_i(x) := V_u(x) - V_d(x)$$

$$V_{ci}(x) := \max \left(0.6\text{psi} \cdot \lambda \cdot \sqrt{\frac{f_{c28}}{\text{psi}}} \cdot b_w \cdot d_c + V_d(x) + \frac{V_i(x) \cdot M_{cre}(x)}{M_{max}(x)}, 1.7\text{psi} \cdot \lambda \cdot \sqrt{\frac{f_{c28}}{\text{psi}}} \cdot b_w \cdot d_c \right)$$

$$V_{cw}(x) := \left(3.5\lambda \cdot \text{psi} \cdot \sqrt{\frac{f_{c28}}{\text{psi}}} + 0.3 \cdot f_{pc}(x) \right) \cdot b_w \cdot d \quad V_c(x) := V_{ci}(x) + V_{cw}(x)$$

$$V_s(x) := \frac{2A_v \cdot f_{ym} \cdot d_c}{S}$$

$$\phi V_n(x) := 0.75(V_c(x) + V_s(x))$$

